# LITCHFIELD ENGINEERING

Civil Engineering & Development Services

# TECHNICAL INFORMATION REPORT for the BCH 93<sup>rd</sup> AVENUE PROJECT

Prepared for:

Premium Homes of Mercer Island, LLC P.O. Box 1639 Mercer Island, WA 98040 (206) 724-1072

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Date Issued: March 30, 2021 Date Revised: May 25, 2021



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#### 1. PROJECT OVERVIEW

**Project:** BCH 93rd Short Plat

City of Mercer Island Permit Number:

Tax Parcel No. 258190-0210

**Site Area:** The parcel area is 0.90 acres.

#### **Site Location:**

The site is located in the City of Mercer Island at 7216 93<sup>rd</sup> Avenue SE within the SE ¼ of Section 30, Township 24 North, Range 5 East, W.M. See the vicinity map on page 2.

#### **Existing Adjacent Development:**

Existing development adjacent to the subject site includes the following:

North – Single-Family Residence

East - Single-Family Residence

South - Single-Family Residence

West – 93<sup>rd</sup> Avenue SE

#### **Pre-developed Site Conditions:**

The property is presently developed with a single-family residence with detached garage and a paved driveway that provides access to 93<sup>rd</sup> Avenue SE. Except for the structures and driveway, the site is vegetated with shrubs and trees. The site generally slopes down from the west to the east at an average grade of approximately 16%. There are steep slopes (>40%) located in the eastern portion of the site. Refer to Figure 4 for a depiction of the existing conditions.

#### **Post-developed Site Conditions:**

The development proposal incorporates the construction of on-site and off-site infrastructure to support the future construction of three new single-family residences. The existing structures will be demolished. The lots will be accessed via new driveways extending from 93<sup>rd</sup> Avenue SE. The project site is located within the Mercer Island Drainage Basin. The site is defined by a single drainage basin that discharges to the west. Developed runoff will be collected, detained, and conveyed to the existing conveyance system within 93<sup>rd</sup> Avenue SE. The discharge from the proposed detention tank will be pumped to an onsite catch basin. In order to get gravity flow from the site to the public conveyance system with 93<sup>rd</sup> Avenue SE, approximately 314 feet of the existing conveyance system will need to be replaced. Water quality is not required since the new pollution generating impervious surface is less than 5,000 SF. The pervious landscape areas will be allowed to infiltrate naturally into the native soil.

#### 2. MINIMUM REQUIREMENTS

Flow Chart #1: Flow Chart for Determining Requirements for New Development was utilized to determine which requirements apply to the project. Per Flow Chart #1 all Minimum Requirements apply to the new and replaced hard surfaces and converted vegetation areas. Please refer to page #4 for Flow Chart #1.

#### Minimum Requirements #1-9:

Minimum Requirement No. 1 – Preparation of Stormwater Site Plans
A Stormwater Site Plan has been prepared for review by the City.

Minimum Requirement No. 2 – Construction Stormwater Pollution Prevention (SWPP) A SWPP (i.e. TESC) plan is included in the project submittal.

Minimum Requirement No. 3 – Source Control of Pollution

Proposed construction source control measures include silt fence and temporary and permanent seeding. Operational and structural BMPs are not proposed. Please refer to Section 3: Construction Stormwater Pollution Prevention Plan for additional BMPs.

Minimum Requirement No. 4 – Preservation of Natural Drainage Systems and Outfalls

The natural drainage pattern and discharges from the site will be maintained to the maximum extent practicable. No significant adverse impacts to the downstream system are expected or anticipated.

#### Minimum Requirement No. 5 – On-site Stormwater Management

Flow Chart #2 Flow Chart for Determining LID MR #5 Requirements was utilized to determine the requirements to meet On-site Stormwater Management. Per Flow Chart #2, List #2 was used to determine the On-site Stormwater Management BMPs feasible for the project. Please refer to Flow Chart #2 on page 5.

#### List #2 Analysis:

Per Section 2.5.5 of the Stormwater Management Manual for Western Washington, the BMPs must be considered in the order listed in List #2 for each surface. The first BMP considered feasible must be implemented to the maximum extent feasible. Below is the feasibility evaluation of the BMPs in the order listed.

#### Lawn and Landscaped areas:

1. **Post Construction Soil Quality and Depth** – This BMP is feasible and will be implemented per BMP T5.13 for all disturbed and converted vegetated areas that are sloped at less than 33%.

#### Roofs:

- 1. Full Dispersion Infeasible due to steep slopes and lack of vegetated flow paths.
- 2. Rain Gardens or Bioretention Infeasible; per city mapping the site is labeled as "non-infiltrating".

- 3. Downspout Dispersion Systems Infeasible due to steep slopes and lack of vegetated flow paths.
- 4. Perforated Stub-out Connection Infeasible; per city mapping the site is labeled as "non-infiltrating".

#### Other Hard Surfaces:

- 1. Full Dispersion Infeasible due to steep slopes and lack of vegetated flow paths.
- 2. Permeable Pavement Infeasible; per city mapping the site is labeled as "non-infiltrating".

#### Minimum Requirement No. 6 - Runoff Treatment

The pollution-generating impervious surfaces (4,584 SF) are less than 5,000 SF therefore water quality treatment is not required.

#### Minimum Requirement No. 7 – Flow Control

Per Section 2.5.7 of the SWMMWW a formal flow control facility is required if the following thresholds are exceeded;

- the total effective impervious surface is 10,000 square feet or more
- ¾ acres or more of native vegetation converted to lawn or landscape, or 2.5 acres or more of native vegetation converted to pasture
- A 0.15 cubic feet per second increase in the 100-year flow frequency

Since the project is proposing greater than 10,000 square feet of effective impervious surface a formal flow control facility is required. The site was analyzed using the Western Washington Hydrology Model (WWHM) provided by the Department of Ecology (DOE). The project site is located in a Level 2 Flow Control Area, therefore forested conditions have been used for the predeveloped modeling analysis. The hydrologic analysis of the site was completed in order to determine the on-site detention necessary to account for the increase in the peak storm release rate for the developed site. Please note for the purpose of this analysis only the developable area of the site was used, as the steep slope areas and associated buffers will not be disturbed. As discussed, given the topography of the existing project site, a single drainage basin was analyzed for the project. Per Appendix III – C: Washington State Department of Ecology Low Impact Development Flow Modeling Guidance, C-9 Soil Quality and Depth, pervious areas that meet the requirements for BMP T5.13 may be modeled as pasture. Therefore, the pervious areas to be amended per BMP T5.13 have been modeled as pasture. The site basin criterion is summarized below. Refer to Appendix A for a detailed depiction of the project's WWHM modeling areas.

#### **WWHM Area Summary:**

#### **Existing Conditions:**

Area Forest		=	0.64 acre
	Total Site	=	0.64 acre

#### **Developed Conditions:**

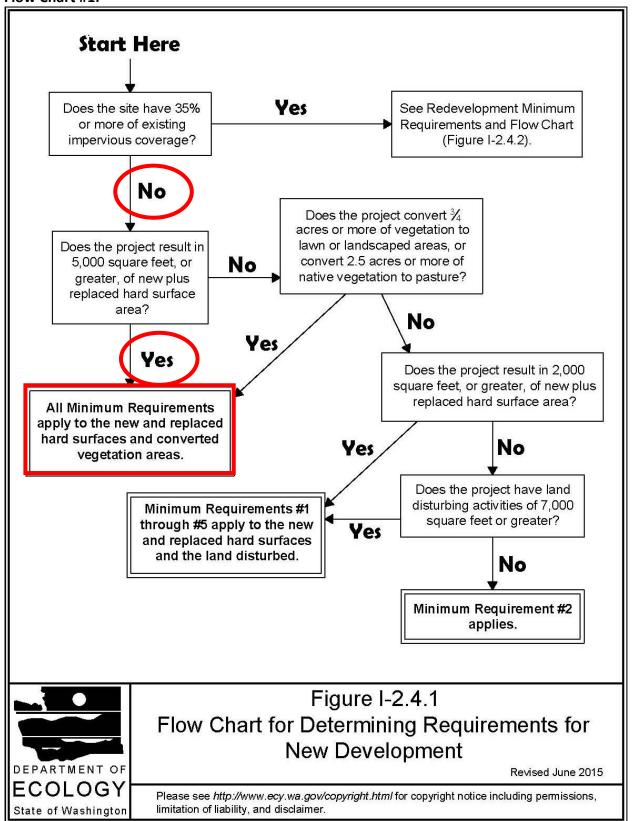
Impervious	=	0.33 acre
Pervious (Lawn/Pasture)	=	0.31 acre
Total Site	=	0.64 acre

The detention tank system was designed for the drainage basin using WWHM. The proposed detention tank is 67 LF and 10' diameter. Please see Appendix A for the complete WWHM tank analysis. The discharge from the proposed detention tank will be pumped to an onsite catch basin located in the northwest corner of the site. The catch basin will convey runoff via gravity to the existing public conveyance system within 93<sup>rd</sup> Ave SE. In order to provide gravity flow, approximately 314 feet of the existing conveyance system will need to be replaced.

Minimum Requirement No. 8 – Wetlands Protection N/A – The project does not discharge to a wetland.

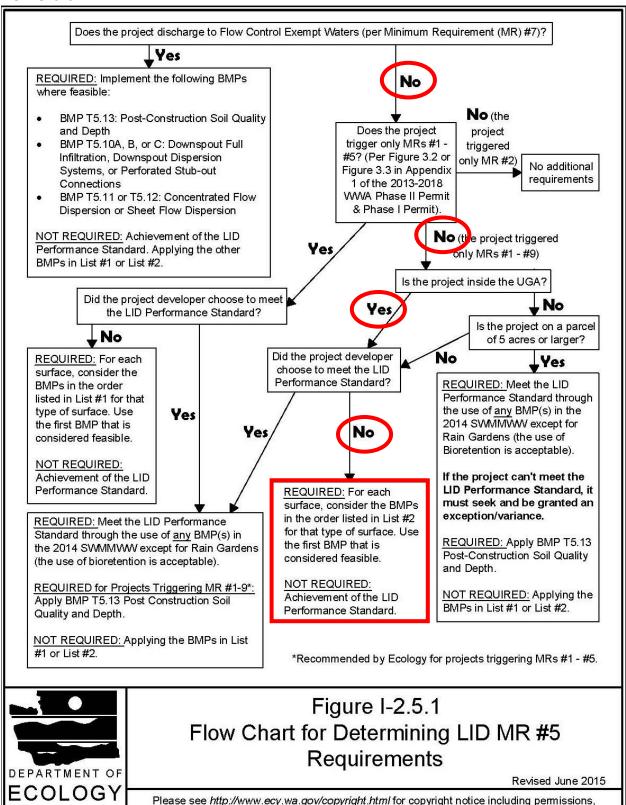
Minimum Requirement No. 9 – Operations and Maintenance A draft Operations and Maintenance Manual is included in Appendix B.

#### Flow Chart #1:



#### Flow Chart #2:

State of Washington



limitation of liability, and disclaimer.

#### 3. CONSTRUCTION STORMWATER POLLUTION PREVENTION PLAN

The project SWPPP addresses the 13 required elements as follows:

Element 1 – Preserve Vegetation/Mark Clearing Limits – Clearing limits will be delineated with silt fence and orange construction fencing.

Element 2 – Establish Construction Access - A quarry spall construction entrance and a wheel wash will be provided if warranted.

Element 3 – Control Flow Rates – The proposed detention tank will be used during construction as a sediment and flow control device. Upon competition of the project, the tank and associated catch basins to be flushed and cleaned of debris.

Element 4 – Install Sediment Controls – Silt fencing will be constructed and is expected to provide construction stormwater sediment control during construction.

Element 5 – Stabilize Soils – Stockpiled or unworked soils will be protected during construction by covering with plastic or temporary or permanent seeding. All exposed soils will be landscaped or seeded and BMP T5.13- Post Construction Soil Amendment will be implemented at the conclusion of the project.

Element 6 – Protect Slopes – Areas of cut slopes to be covered with plastic per BMP C123 until permanently stabilized.

Element 7 – Protect Drain Inlets – The existing and newly constructed conveyance system inlets in the vicinity of the project site will be protected with catch basin filters during construction.

Element 8 – Stabilize Channels and Outlets – There are no existing or proposed surface channels or outfalls. Therefore the use of typical energy dissipation devices and channel lining such as riprap are not anticipated.

Element 9 – Control Pollutants – The small size of this project will limit the opportunity for discharge of pollutants. Waste/demolition debris will not be stockpiled, fueling will be done offsite and concrete trucks will be washed out off-site.

Element 10 – Control De-watering – De-watering is not anticipated.

Element 11 – Maintain BMPs – BMPs will be maintained as necessary to assure continued functioning.

Element 12 – Manage the Project – An inspector (sites less than 1 acre) will be present or on call to ensure BMPs are maintained and assess effectiveness of ESC measures. Rainy season requirements will be implemented if necessary.

Element 13 – Protect LID BMPs – N/A. No LID BMPs are proposed.

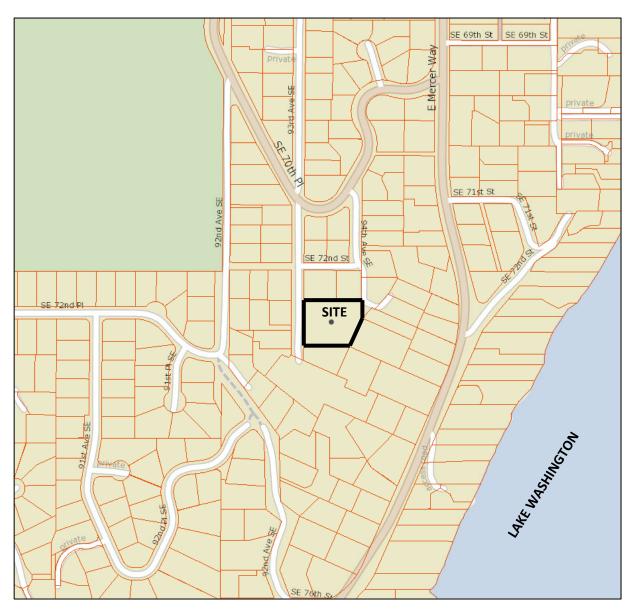


Figure 1 – Vicinity Map

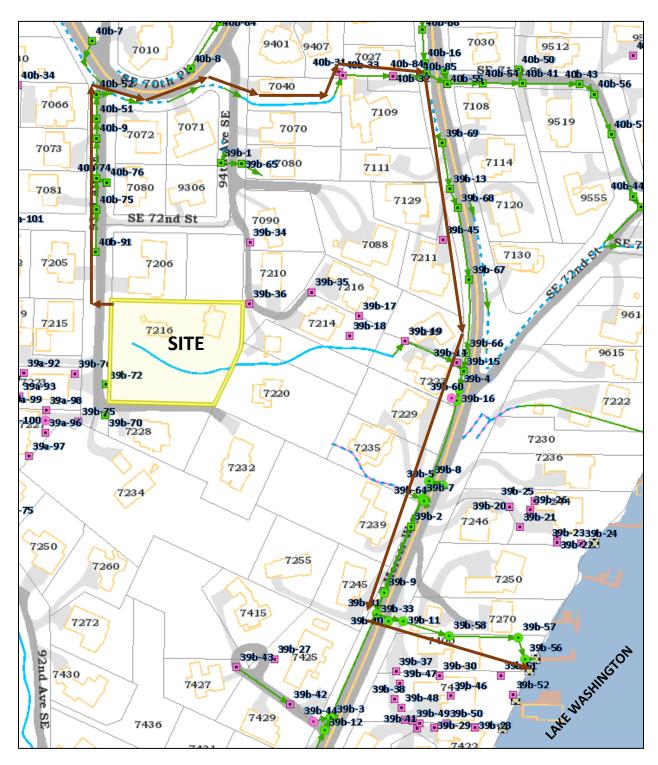


Figure 2 - Downstream Map

# Appendix A WWHM OUTPUT

# WWHM2012 PROJECT REPORT

# General Model Information

Project Name: BCH 93rd Short Plat Tank 5-17-21

Site Name: BCH 93rd Short Plat Site Address: 7216 93rd Ave SE

City: Mercer Island
Report Date: 5/18/2021
Gage: Seatac

Data Start: 1948/10/01
Data End: 2009/09/30
Timestep: 15 Minute
Precip Scale: 1.000

Version Date: 2019/09/13

Version: 4.2.17

#### **POC Thresholds**

Low Flow Threshold for POC1: 50 Percent of the 2 Year

High Flow Threshold for POC1: 50 Year

# Landuse Basin Data Predeveloped Land Use

#### Basin 1

Bypass: No

GroundWater: No

Pervious Land Use acre C, Forest, Steep 0.64

Pervious Total 0.64

Impervious Land Use acre

Impervious Total 0

Basin Total 0.64

Element Flows To:

Surface Interflow Groundwater

### Mitigated Land Use

#### Basin 1

Bypass: No

GroundWater: No

Pervious Land Use acre C, Lawn, Steep 0.02 C, Pasture, Flat 0.05 C, Pasture, Mod 0.07 C, Pasture, Steep 0.17

Pervious Total 0.31

Impervious Land Use acre ROADS FLAT 0.27 ROADS MOD 0.06

Impervious Total 0.33

Basin Total 0.64

Element Flows To:

Surface Interflow Groundwater

Tank 1 Tank 1

# Routing Elements Predeveloped Routing

### Mitigated Routing

#### Tank 1

Dimensions

Depth: 10 ft.
Tank Type: Circular
Diameter: 10 ft.
Length: 67 ft.

Discharge Structure

Riser Height: 9.5 ft. Riser Diameter: 12 in.

Orifice 1 Diameter: 0.5 in. Elevation:0 ft. Orifice 2 Diameter: 0.7812 inElevation:6 ft. Orifice 3 Diameter: 0.4844 inElevation:7 ft.

Element Flows To:

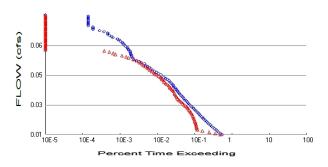
Outlet 1 Outlet 2

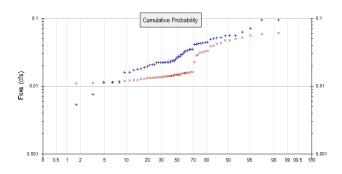
### Tank Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	Infilt(cfs)
0.0000	0.000000	0.000000	0.000	0.000
0.1111	0.003225	0.000239	0.002	0.000
0.2222	0.004535	0.000675	0.003	0.000
0.3333	0.005522	0.001236	0.003	0.000
0.4444	0.006339	0.001896	0.004	0.000
0.5556	0.007046	0.002640	0.005	0.000
0.6667	0.007673	0.003459	0.005	0.000
0.7778	0.008239	0.004343	0.006	0.000
0.8889 1.0000	0.008754 0.009229	0.005288 0.006287	0.006 0.006	0.000 0.000
1.1111	0.009229	0.000287	0.006	0.000
1.2222	0.010076	0.007337	0.007	0.000
1.3333	0.010076	0.009575	0.007	0.000
1.4444	0.010437	0.010757	0.008	0.000
1.5556	0.011149	0.010737	0.008	0.000
1.6667	0.011464	0.013234	0.008	0.000
1.7778	0.011761	0.014525	0.009	0.000
1.8889	0.012041	0.015847	0.009	0.000
2.0000	0.012305	0.017200	0.009	0.000
2.1111	0.012554	0.018581	0.009	0.000
2.2222	0.012789	0.019989	0.010	0.000
2.3333	0.013011	0.021422	0.010	0.000
2.4444	0.013220	0.022880	0.010	0.000
2.5556	0.013418	0.024360	0.010	0.000
2.6667	0.013604	0.025861	0.011	0.000
2.7778	0.013778	0.027382	0.011	0.000
2.8889	0.013943	0.028923	0.011	0.000
3.0000	0.014097	0.030480	0.011	0.000
3.1111	0.014241	0.032055	0.012	0.000
3.2222	0.014376	0.033645	0.012	0.000
3.3333	0.014501	0.035249	0.012	0.000
3.4444	0.014618	0.036867	0.012	0.000
3.5556	0.014725	0.038497	0.012	0.000
3.6667 3.7778	0.014824 0.014914	0.040139 0.041791	0.013 0.013	0.000 0.000
3.8889	0.014914	0.041791	0.013	0.000
3.0009	0.014990	0.043433	0.013	0.000

4.0000 4.1111 4.2222 4.3333 4.4444 4.5556 4.6667 4.7778 4.8889 5.0000 5.1111 5.2222 5.3333 5.4444 5.5556 5.6667 5.7778 5.8889 6.0000 6.1111 6.2222 6.3333 6.4444 6.5556 6.6667 6.7778 6.8889 7.0000 7.1111 7.2222 7.3333 7.4444 7.5556 7.6667 7.7778 7.8889 8.0000 8.1111 8.2222 8.3333 8.4444 8.5556 8.6667 7.7778 7.8889 8.0000 8.1111 8.2222 8.3333 8.4444 8.5556 8.6667 8.7778 8.8889 9.0000 9.1111 9.2222 9.3333 9.4444 9.5556 8.6667	0.015070 0.015136 0.015194 0.015244 0.015286 0.015320 0.015347 0.015366 0.015377 0.015381 0.015377 0.015366 0.015347 0.015320 0.015347 0.015320 0.015244 0.015136 0.015244 0.015194 0.015136 0.015070 0.014996 0.014914 0.014824 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014725 0.014618 0.014761 0.01464 0.011761 0.011761 0.011761 0.011761 0.011761 0.011761 0.011761 0.01076 0.009668 0.009229 0.008754 0.008239 0.007673 0.007673 0.0076339	0.045123 0.046802 0.048487 0.050178 0.051874 0.053574 0.055278 0.056984 0.058693 0.060401 0.062110 0.063818 0.065525 0.067228 0.068929 0.070625 0.072316 0.074001 0.075679 0.077350 0.079012 0.080664 0.082305 0.083936 0.085554 0.087158 0.0887158 0.088748 0.090322 0.091880 0.094942 0.096443 0.093420 0.094942 0.096443 0.097923 0.099380 0.100814 0.102222 0.103603 0.104956 0.105769 0.108825 0.110466 0.111228 0.112368 0.113466 0.114516 0.117344 0.118163 0.118907 0.118667	0.013 0.013 0.014 0.014 0.014 0.014 0.015 0.015 0.015 0.015 0.015 0.016 0.016 0.016 0.016 0.022 0.024 0.026 0.028 0.029 0.031 0.032 0.033 0.034 0.037 0.039 0.041 0.042 0.044 0.045 0.045 0.046 0.047 0.049 0.050 0.051 0.052 0.053 0.051 0.052 0.053 0.051 0.052 0.053 0.055 0.055 0.055 0.056 0.057 0.058 0.059 0.060 0.061 0.061	0.000 0.000
9.3333	0.007673	0.117344	0.060	0.000
9.4444	0.007046	0.118163	0.061	

# Analysis Results POC 1





+ Predeveloped x Mitigated

Predeveloped Landuse Totals for POC #1

Total Pervious Area: 0.64
Total Impervious Area: 0

Mitigated Landuse Totals for POC #1 Total Pervious Area: 0.31 Total Impervious Area: 0.33

Flow Frequency Method: Log Pearson Type III 17B

Flow Frequency Return Periods for Predeveloped. POC #1

 Return Period
 Flow(cfs)

 2 year
 0.028639

 5 year
 0.045619

 10 year
 0.056644

 25 year
 0.069977

 50 year
 0.079394

 100 year
 0.088348

Flow Frequency Return Periods for Mitigated. POC #1

 Return Period
 Flow(cfs)

 2 year
 0.017631

 5 year
 0.028354

 10 year
 0.037696

 25 year
 0.052559

 50 year
 0.066207

 100 year
 0.082389

#### **Annual Peaks**

Annual Peaks for Predeveloped and Mitigated. POC #1

Year	Predeveloped	Mitigated
1949	0.032	0.013
1950	0.035	0.016
1951	0.056	0.052
1952	0.021	0.012
1953	0.016	0.012
1954	0.022	0.015
1955	0.041	0.016
1956	0.031	0.023
1957	0.029	0.014
1958	0.028	0.015

# Ranked Annual Peaks

Ranked Annual Peaks for Predeveloped and Mitigated. POC #1

Rank	Predeveloped	
1	0.0947	0.0608
2	0.0945	0.0582
3	0.0715	0.0564

# **Duration Flows**

The Facility PASSED

Flow(cfs) 0.0143 0.0150	Predev 11424 10371	<b>Mit</b> 10307 7602	Percentage 90 73	Pass/Fail Pass Pass
0.0156	9441	5227	55	Pass
0.0163	8630	3418	39	Pass
0.0169 0.0176	7948 7279	2513 2464	31 33	Pass Pass
0.0176	6671	2417	36	Pass
0.0189	6098	2372	38	Pass
0.0196	5612	2331	41	Pass
0.0202	5153	2291	44	Pass
0.0209 0.0216	4770 4408	2239 2192	46 49	Pass Pass
0.0210	4072	2147	52	Pass
0.0229	3767	2072	55	Pass
0.0235	3542	2013	56	Pass
0.0242	3292	1943	59	Pass
0.0248 0.0255	3071 2862	1881 1809	61 63	Pass Pass
0.0253	2667	1735	65	Pass
0.0268	2479	1661	67	Pass
0.0275	2306	1572	68	Pass
0.0281	2158	1489	68	Pass
0.0288 0.0294	1970 1828	1403 1315	71 71	Pass Pass
0.0301	1684	1218	72	Pass
0.0308	1572	1111	70	Pass
0.0314	1461	1027	70	Pass
0.0321 0.0327	1362 1268	947 867	69	Pass
0.0327	1173	808	68 68	Pass Pass
0.0340	1102	753	68	Pass
0.0347	1030	711	69	Pass
0.0354	962	688	71	Pass
0.0360 0.0367	905 849	659 634	72 74	Pass Pass
0.0373	802	614	7 <del>4</del> 76	Pass
0.0380	750	585	78	Pass
0.0386	715	548	76	Pass
0.0393	679 638	508 477	74 74	Pass
0.0400 0.0406	605	477 454	74 75	Pass Pass
0.0413	572	429	75 75	Pass
0.0419	542	389	71	Pass
0.0426	503	355	70	Pass
0.0432 0.0439	469 435	327 310	69 71	Pass Pass
0.0446	391	286	73	Pass
0.0452	351	272	77	Pass
0.0459	321	253	78	Pass
0.0465	293	232	79 76	Pass
0.0472 0.0478	264 230	203 180	76 78	Pass Pass
0.0485	203	167	82	Pass

0.0649       25       0       0       Pa         0.0656       22       0       0       0       Pa         0.0662       20       0       0       0       Pa         0.0669       18       0       0       Pa         0.0676       14       0       0       Pa         0.0682       13       0       0       Pa         0.0689       11       0       0       Pa         0.0702       10       0       0       Pa         0.0708       8       0       0       Pa         0.0715       6       0       0       Pa         0.0722       4       0       0       Pa         0.0728       4       0       0       Pa         0.0741       3       0       0       Pa         0.0748       3       0       0       Pa         0.0768       3       0       0       Pa         0.0774       3       0       0       Pa         0.0787       3       0       0       Pa         0.0787       3       0       0       Pa
---

# Water Quality

Water Quality
Water Quality BMP Flow and Volume for POC #1
On-line facility volume: 0 acre-feet
On-line facility target flow: 0 cfs.
Adjusted for 15 min: 0 cfs.
Off-line facility target flow: 0 cfs.
Adjusted for 15 min: 0 cfs.

# LID Report

LID Technique	Used for Treatment?	Total Volume Needs Treatment (ac-ft)		Volume	Cumulative Volume Infiltration Credit	Percent Volume Infiltrated	Water Quality	Percent Water Quality Treated	Comment
Tank 1 POC		63.13				0.00			
Total Volume Infiltrated		63.13	0.00	0.00		0.00	0.00	0%	No Treat. Credit
Compliance with LID Standard 8% of 2-yr to 50% of 2-yr									Duration Analysis Result = Passed

# Model Default Modifications

Total of 0 changes have been made.

# PERLND Changes

No PERLND changes have been made.

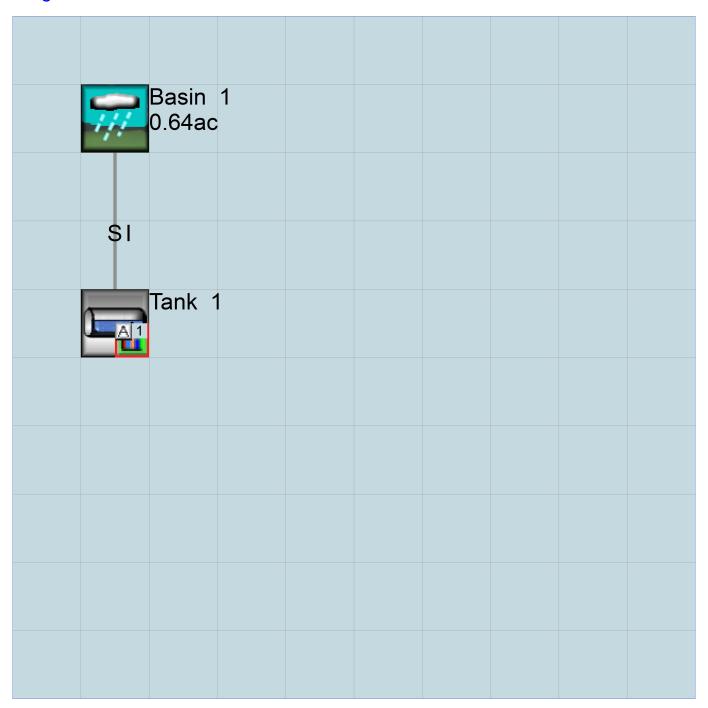
# **IMPLND Changes**

No IMPLND changes have been made.

# Appendix Predeveloped Schematic

Basin 0.64ac	1			

# Mitigated Schematic



## Disclaimer

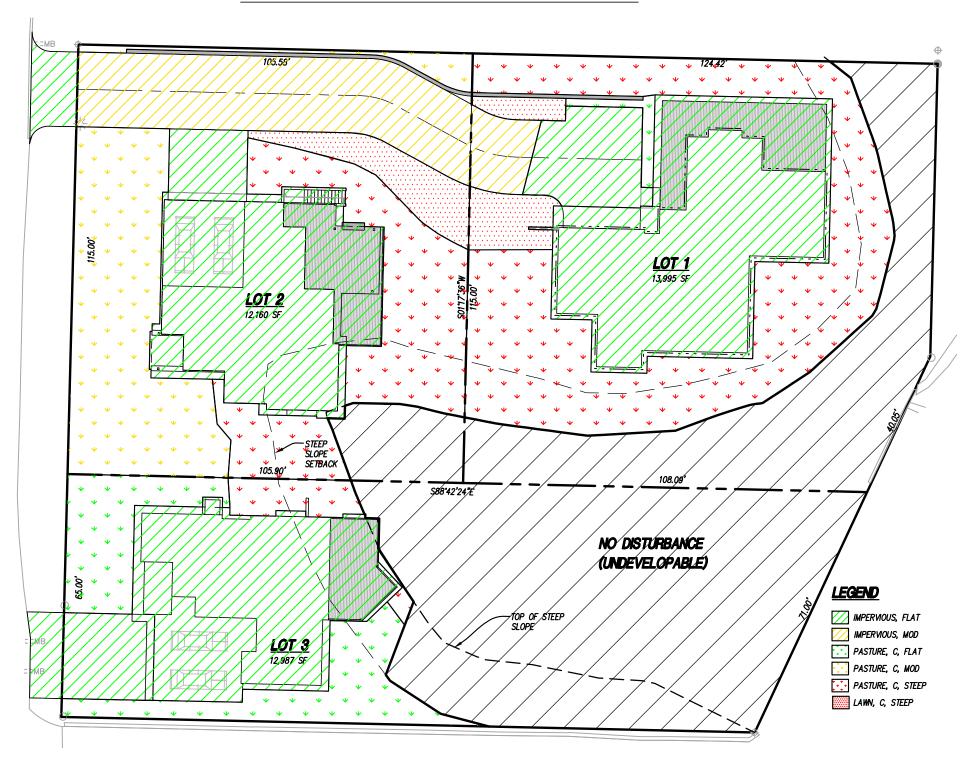
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#### **BCH 93rd SHORT PLAT MODELING AREAS EXHIBIT**



# Appendix B MAINTENANCE & OPERATIONS MANUAL

Table V-4.5.2(3) Maintenance Standards - Closed Detention Systems (Tanks/Vaults)

Maintenance Component		Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Storage Area	Plugged Air Vents	One-half of the cross section of a vent is blocked at any point or the vent is damaged.	Vents open and functioning.
	Debris and Sediment	Accumulated sediment depth exceeds 10% of the diameter of the storage area for 1/2 length of storage vault or any point depth exceeds 15% of diameter.  (Example: 72-inch storage tank would require cleaning when sediment reaches depth of 7 inches for more than 1/2 length of tank.)	All sediment and debris removed from storage area.
	Joints Between Tank/Pipe Section	Any openings or voids allowing material to be transported into facility.  (Will require engineering analysis to determine structural stability).	All joint between tank/pipe sections are sealed.
	Tank Pipe Bent Out of Shape	Any part of tank/pipe is bent out of shape more than 10% of its design shape. (Review required by engineer to determine structural stability).	Tank/pipe repaired or replaced to design.
Vault Structure Includes Cracks in Wall, Bottom, Damage to Frame and/or Top Slab  entering the structure through the cracks, or maintenance/inspection personnel determines the structurally sound.  Cracks wider than 1/2-inch at the joint of any inlet		maintenance/inspection personnel determines that the vault is not structurally sound.  Cracks wider than 1/2-inch at the joint of any inlet/outlet pipe or any	Vault replaced or repaired to design specifications and is structurally sound.  No cracks more than 1/4-inch wide at the joint of the inlet/outlet pipe.

Maintenance Component	Defect	Defect Conditions When Maintenance is Needed	
	Cover Not in Place	Cover is missing or only partially in place. Any open manhole requires maintenance.	Manhole is closed.
Manhole	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than 1/2 inch of thread (may not apply to self-locking lids).	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure. Intent is to keep cover from sealing off access to maintenance.	Cover can be removed and reinstalled by one maintenance person.
	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, misalignment, not securely attached to structure wall, rust, or cracks.	Ladder meets design standards. Allows maintenance person safe access.
Catch Basins	See "Catch Basins" (No. 5)	See "Catch Basins" (No. 5).	See "Catch Basins" (No. 5).

Table V-4.5.2(4) Maintenance Standards - Control Structure/Flow Restrictor

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed
General		Material exceeds 25% of sump depth or 1 foot below orifice plate.	Control structure orifice is not blocked. All trash and debris removed.
	Structural Damage	Structure is not securely attached to manhole wall.  Structure is not in upright position (allow up to 10% from plumb).  Connections to outlet pipe are not watertight and show signs of rust.  Any holes - other than designed holes - in the structure.	Structure securely attached to wall and outlet pipe.  Structure in correct position.  Connections to outlet pipe are water tight; structure repaired or replaced and works as designed.  Structure has no holes other than designed holes.
Cleanout Gate	Damaged or Missing	Cleanout gate is not watertight or is missing.  Gate cannot be moved up and down by one maintenance person.  Chain/rod leading to gate is missing or damaged.  Gate is rusted over 50% of its surface area.	Gate is watertight and works as designed.  Gate moves up and down easily and is watertight.  Chain is in place and works as designed.  Gate is repaired or replaced to meet design standards.
Orifice Plate	Damaged or Missing	Control device is not working properly due to missing, out of place, or bent orifice plate.	Plate is in place and works as designed.

Maintenance Component	Defect	Condition When Maintenance is Needed	Results Expected When Maintenance is Performed	
	Obstructions	Any trash, debris, sediment, or vegetation blocking the plate.	Plate is free of all obstructions and works as designed.	
Overflow Pipe	Obstructions	Any trash or debris blocking (or having the potential of blocking) the overflow pipe.	Pipe is free of all obstructions and works as designed.	
and the second second	See "Closed Detention Systems" (No. 3).	See "Closed Detention Systems" (No. 3).	See "Closed Detention Systems" (No. 3).	
Catch Basin	See "Catch Basins" (No. 5).	See "Catch Basins" (No. 5).	See "Catch Basins" (No. 5).	

Table V-4.5.2(5) Maintenance Standards - Catch Basins

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is performed
	Trash & Debris	Trash or debris (in the basin) that exceeds 60 percent of the sump depth as measured from the bottom of basin to invert of the lowest pipe into or out of the basin, but in no case less than a minimum of six inches clearance from the debris surface to the invert of the lowest pipe.  Trash or debris in any inlet or outlet pipe blocking more than 1/3 of its height.  Dead animals or vegetation that could generate odors that could cause complaints or dangerous gases (e.g., methane).	No Trash or debris located immediately in front of catch basin or on grate opening.  No trash or debris in the catch basin.  Inlet and outlet pipes free of trash or debris.  No dead animals or vegetation present within the catch basin.
	Sediment	Sediment (in the basin) that exceeds 60 percent of the sump depth as measured from the bottom of basin to invert of the lowest pipe into or out of the basin, but in no case less than a minimum of 6 inches clearance from the sediment surface to the invert of the lowest pipe.	No sediment in the catch basin
	Structure Damage to Frame and/or Top Slab	is to make sure no material is running into basin).  Frame not sitting flush on ton slab, i.e., separation of more than 3/4 inch of the	Top slab is free of holes and cracks.  Frame is sitting flush on the riser rings or top slab and firmly attached.

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is performed
	Rottom	Maintenance person judges that structure is unsound.  Grout fillet has separated or cracked wider than 1/2 inch and longer than 1 foot at the joint of any inlet/outlet pipe or any evidence of soil particles entering catch basin through cracks.	Basin replaced or repaired to design standards.  Pipe is regrouted and secure at basin wall.
	Settlement/ Misalignment	If failure of basin has created a safety, function, or design problem.	Basin replaced or repaired to design standards.
	Vegetation	Vegetation growing across and blocking more than 10% of the basin opening.  Vegetation growing in inlet/outlet pipe joints that is more than six inches tall and less than six inches apart.	No vegetation blocking opening to basin.  No vegetation or root growth present.
	Contamination and Pollution	See "Detention Ponds" (No. 1).	No pollution present.
	Cover Not in Place	Cover is missing or only partially in place. Any open catch basin requires maintenance.	Catch basin cover is closed
WAR THE RESERVE OF THE PARTY OF	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than 1/2 inch of thread.	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying normal lifting pressure.  (Intent is keep cover from sealing off access to maintenance.)	Cover can be removed by one maintenance person.
Ladder	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, not securely attached to basin wall, misalignment, rust, cracks, or sharp edges.	Ladder meets design standards and allows maintenance person safe access.
Metal Grates (If Applicable)	Grate opening Unsafe	Grate with opening wider than 7/8 inch.	Grate opening meets design standards.
	Trash and Debris	Trash and debris that is blocking more than 20% of grate surface inletting capacity.	Grate free of trash and debris.
	Damaged or Missing.	Grate missing or broken member(s) of the grate.	Grate is in place and meets design standards.